

SURVEYING

Surveying is normally regarded as the most fundamental base, or more precisely, the art and science of map making. It is defined as an art to determine the relative positions on, above or beneath the surface of the earth, with respect to each other, by measurements of horizontal and vertical distances, angles and directions.

Common Terminologies

1. Level surface - It refers to any surface parallel to the mean spheroidal surface of the earth. Any line contained in the level surface is termed as a level line.

2. Datum - It refers to any arbitrary assumed level surface or line from which vertical distances or elevations are measured.

3. Bench mark - It refers to a fixed reference point of known elevation above sea level. Depending upon the permanency and precision, bench mark may be of four types; viz.,

- a) Great Triangometric survey Bench Mark (G.T.S.B.M.).
- b) Permanent Bench Mark.
- c) Arbitrary Bench Mark.
- d) Temporary Bench Mark.

4. Line of collimation - It is the line passing through the intersection of the cross hairs to the optical centre of the object glass of the telescope.

5. Change point \Rightarrow It denotes the shifting of the leveling instrument. It is a point on which both the fore sight of the former set-up and the back sight of the new set-up are observed.

6. Leveling \Rightarrow leveling may be defined as the art of determining the relative heights or elevations of points or objects on the earth surface. It deals with the measurement in a vertical plane.

7. Back sight \Rightarrow It is a staff reading taken in a point of known elevation, as on a bench mark or a change point. It is called a plus sight. It is the first staff reading taken after the level is set up and levelled.

8. Fore sight \Rightarrow It is a staff reading on a point, whose elevation is to be determined as on a change point. It is also called a minus sight. It is the last staff reading denoting the shifting of the instrument.

9. Intermediate sight \Rightarrow It is a staff reading taken on a point of unknown elevation between back sight and fore sight.

10. Parallax \Rightarrow It is the apparent movement of the image relative to the cross hairs when the image formed by the objective does not fall in the plane of the diaphragm.

11. Traversing \Rightarrow A traverse survey is one in which the frame work consists of a series of connected lines, the lengths and directions of which are measured with a chain or tape and with an angular instrument respectively.

A traverse may be classified as - a) closed traverse, and
b) open traverse.

12. Fore & Back bearings → Every line has two bearings, one observed at each end of the line. The bearing of a line in the direction of the

progress of the survey is called the 'Fore or Forward bearing' (F.B.), while its bearing in the opposite direction is known as the 'Back bearing' (B.B.). The fore and back bearings of a line differ exactly by 180° .

13. Closing error → It is the distance by which the end of a traverse falls short in coinciding with the starting point of the traverse.

14. Local attraction → It refers to the deflection of a magnetic needle caused by external disturbances induced by the proximity of magnetic substances.

15. Contouring → The elevations and depressions of the surface of the ground are shown on a map by means of contour lines. A contour may be defined as the line of intersection of a level surface with the surface of the ground.

All points on any one contour have the same elevation above the datum surface. The line joining these points on the map is called 'contour lines' or 'contours'. The vertical distance between two consecutive contours is called the 'contour interval', and the horizontal distance between any two consecutive contours is termed as 'horizontal equivalent'.

The more broken the ground, the greater must be the contour interval; otherwise the contours will be close to each other. If the interval is smaller the amount of field work is greater.

Common Equipments of Surveying :-

1. Transit Theodolite :- It is the most accurate instrument by which horizontal and vertical angles can be determined with great precision. The size of it varies from 8 to 25 cm. Sometimes, a compass is fitted on the theodolite to note the bearings. The instrument is mounted on a tripod made of solid logs.

2. Prismatic Compass :- It is used in wooded country rough traverses, filling in details preliminary survey for a road, military purposes, such as, sketching and night marching etc. It is unreliable in places abounding in magnetic rock or iron ore. It is less accurate than theodolite.

3. Dumpy Level :- This is the most widely used direct leveling instrument. It consists of a telescope which is rigidly fixed to its support. It can neither be rotated about its longitudinal axis nor can it be removed from its support. It is very advantageous when several observations to be made with one set up of the instrument.

Other common accessories are -

4. Levelling Staff :- Made up of steel or aluminium and may have graduations either in feet & metre ; **5. Tape :-** Used in measuring short lengths ;

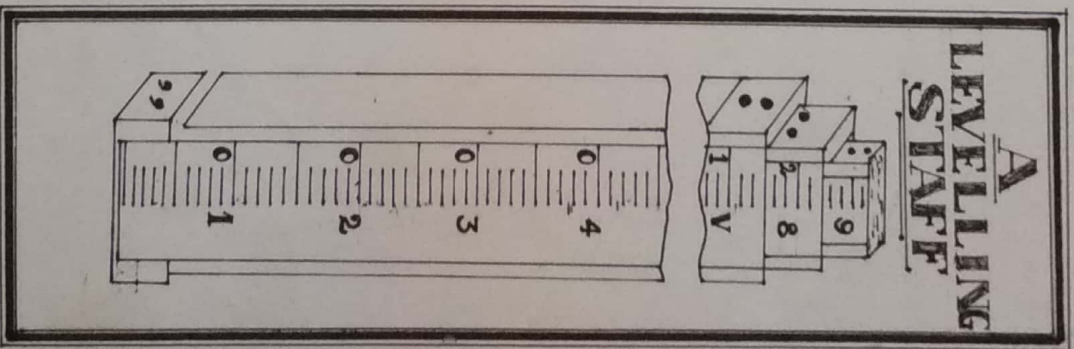
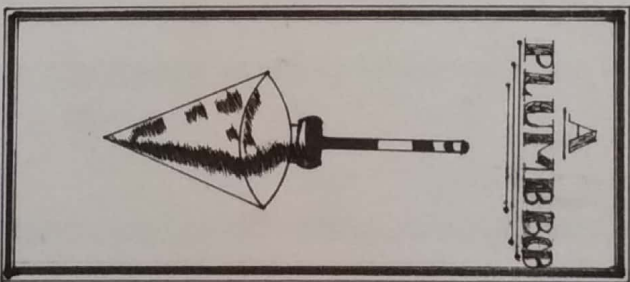
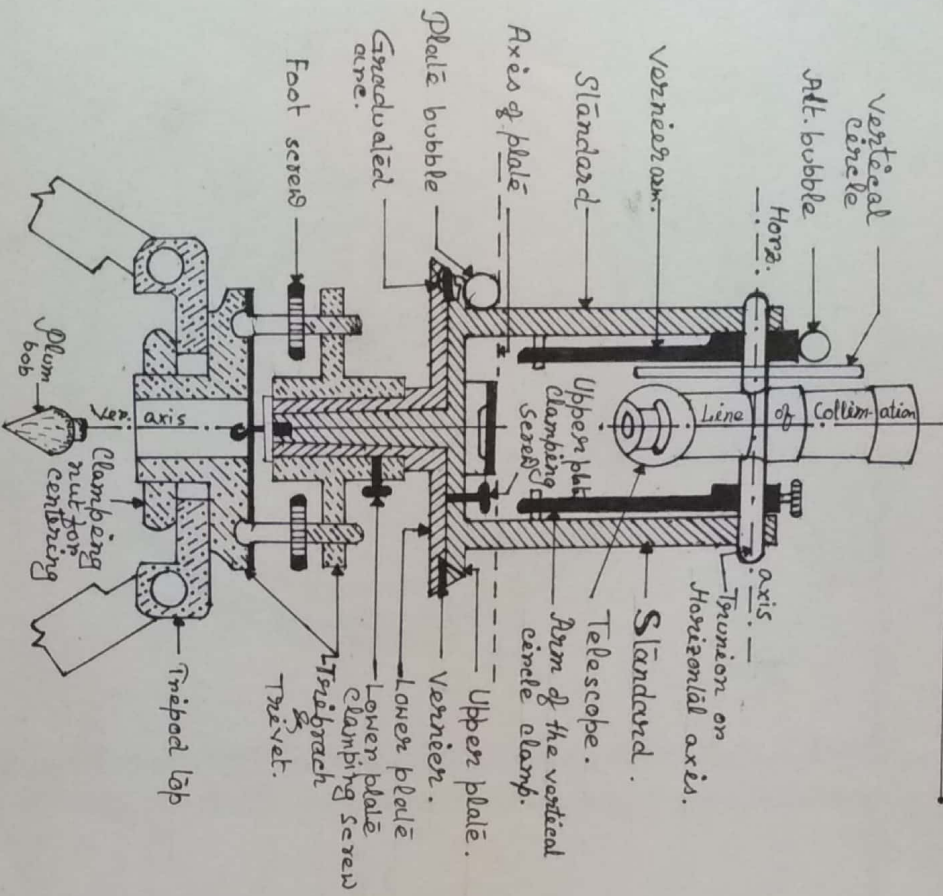
6. Pin :- Used in chaining and in marking the stations with a tag on it ;

7. Ranging Rod :- Used in station marking and setting out lines ;

8. Plumbob :- Used mainly for accurate centering ; etc.

EQUIPMENTS OF THEODOLITE SURVEY

TRANSIT THEODOLITE

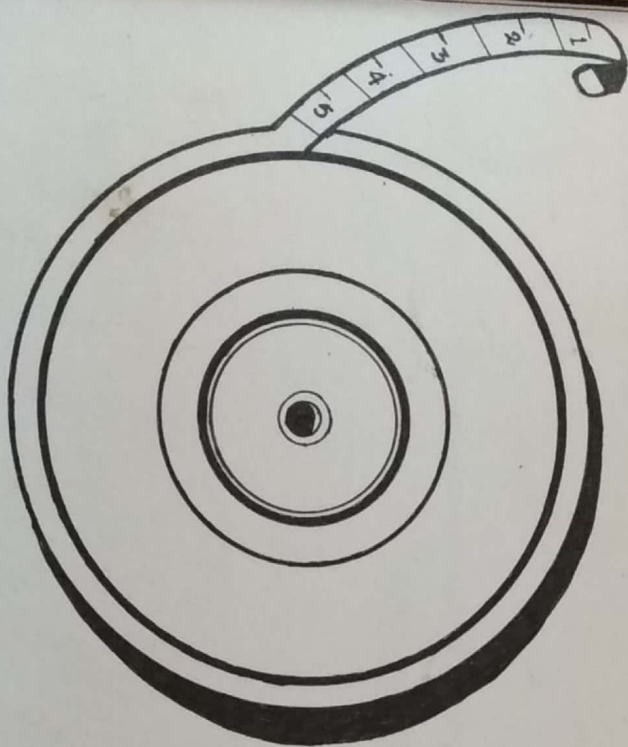


Quesada
22-23

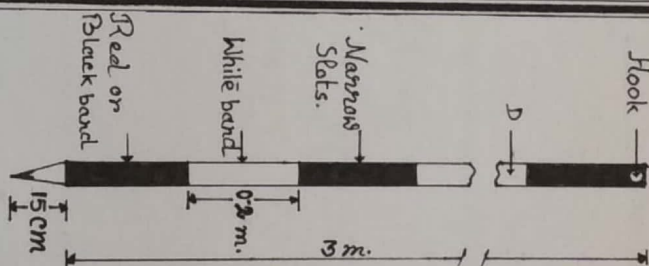
Contd. →

EQUIPMENTS OF THEODOLITE SURVEY

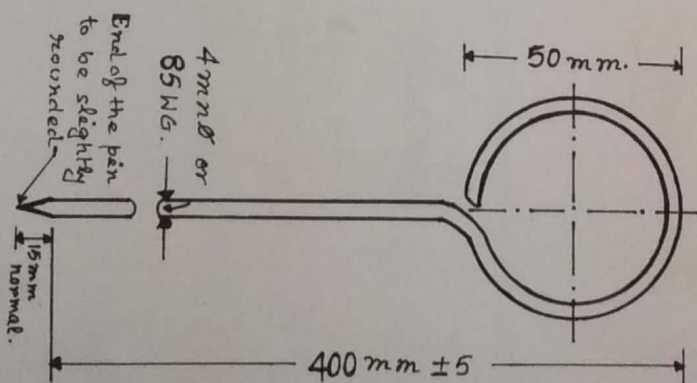
A MEASURING TAPE



A RAINING ROD



A GROUND PIN



over
2.9.20

DETERMINATION OF HEIGHT & DISTANCE BY

TRANSIT THEODOLITE

Base : Accessible

*Date : 1/9/2000.

*Place : Chandernagore College ground.

*Time : 2 PM to 3 PM.

*Instrument No : 55607.

*Object

: Telephone tower.

*Surveyor

: Roll No. : 404/AH-010027.
(Group-A).

FACE	VERNIER READING			REMARKS
	VC	VD	MEAN	
LEFT	18° 28'	18° 30'	18° 29'	1. Instrumental height = 1.330 metre 2. Known distance = 13.50 metre.
RIGHT	18° 30'	18° 34'	18° 32'	
GRAND MEAN ANGLE	—	—	18° 30' 30"	

Quesada
22.9.2000

CALCULATION HEIGHT & DISTANCE BASE : ACCESSIBLE

1. $\angle \theta = 18^\circ 30' 30''$

2. The instrumental height on the height of the collimation, $h_2 = 1.33$ metre.

3. CALCULATION OF GROUND DISTANCE:-

* A. By Stadia method \rightarrow

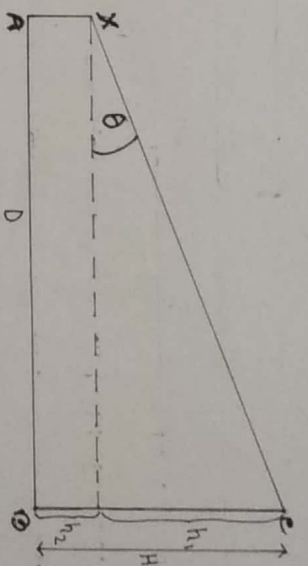
Total ground distance, $D = (\text{Lower stadia} - \text{Upper stadia}) \times 100$
 $= \{(1.395 - 1.260) \times 100\}$ metre.
 $= 13.50$ metre.

* B. By 1st elevation method \rightarrow

Total ground distance, $D = (\text{Reading at } 1^\circ - \text{Reading at } 0^\circ) \times \cot 1^\circ$
 $= \{(1.565 - 1.330) \times \cot 1^\circ\}$ metre ; $= 13.46$ metre.

4. Height of the object above the line of collimation $h_1 = D \tan \theta$
 $= \{13.50 \times \tan 18^\circ 30' 30''\}$ metre
 $= \{13.50 \times 0.334757054\}$ metre
 $= 4.519$ metre.

5. Height of the object above the ground surface, $H = (h_1 + h_2) = (4.519 + 1.330)$ metre
 $= 5.85$ metre.



HEIGHT & DISTANCE

By

TRANSIT THEODOLITE

Base : Accessible

DATE : 28/8/2000.

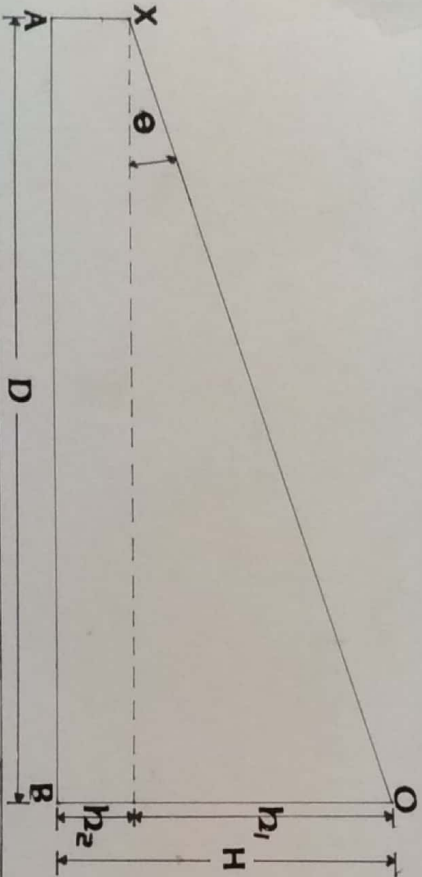
PLACE : CHANDERNAGORE COLLEGE GROUND.

TIME : 12.00 NOON TO 1.40 PM.

INSTRUMENT NO.: 55607.

OBJECT : TELEPHONE TOWER.

SURVEYOR : ROLL NO. 404/AH-010027.
(GROUP-A).



SCALE
Horizontal
Vertical
1 : 100.
metre.

★ Index

h_1 ⇒ Height of the Object above the
Line of Collimation.

h_2 ⇒ Height of the Collimation.

H ⇒ Height of the Object above
Ground Surface.

D ⇒ Total ground distance.

$\angle \theta$ ⇒ $18^\circ 30' 30''$.

Corrected

DETERMINATION OF HEIGHT & DISTANCEBYTRANSIT THEODOLITEBase : Inaccessible

Date : 28/8/2000.

Place : CHANDERNAGORE COLLEGE GROUND.

Time : 12.00 NOON. TO 1.40 P.M.

Instrument NO : 55607.

Object : RAIN WATER PIPE.

Surveyor : ROLL NO. : 404/AH-010027.
(GROUP-A).

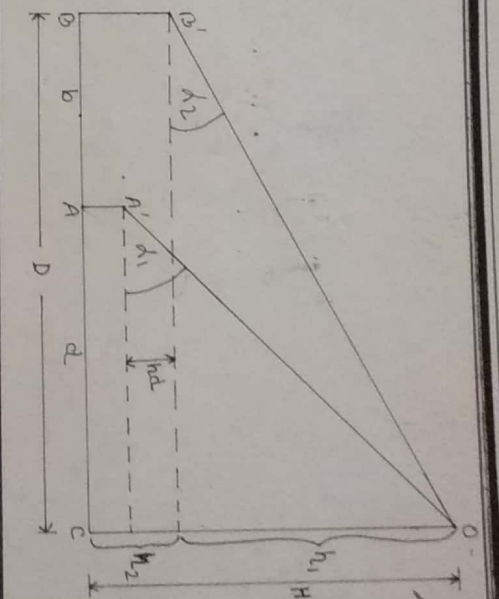
STATION	FACE	VERNIER READING			REMARKS
		VC	VD	MEAN	
Station - 'A'	Left	32° 46'	32° 45'	32° 45' 30"	1. Instrumental height at Station $A = 1.38 \text{ metre}$ 2. Instrumental height at Station $B = 1.45 \text{ metre}$ 3. Known distance = 6.20 metre. <i>Over</i> <i>22.9.2000</i>
	Right	32° 40'	32° 40'	32° 40'	
	GRAND MEAN ANGLE	—	—	32° 42' 45"	
	Left	25° 29'	25° 30'	25° 29' 30"	
Station - 'B'	Right	25° 25'	25° 26'	25° 25' 30"	
	GRAND MEAN ANGLE	—	—	25° 27' 30"	

CALCULATION OF

HEIGHT & DISTANCE

Case: Inaccessible.

ALG



1. Known distance, $b = 6.20$ metre.

2. $\angle \alpha_1 = 32^\circ 42' 45''$; $\angle \alpha_2 = 25^\circ 27' 30''$

3. Height of the instrument at station-A = 1.38 metre.
Height of the instrument at station-D = 1.45 metre.

\therefore The collimation difference, $h_2 = (1.45 - 1.38)$ metre ; = 0.07 metre.

4. Unknown distance, $d = \frac{(b + h_2 \cot \alpha_2) \tan \alpha_2}{(\tan \alpha_1 - \tan \alpha_2)}$

$$= \frac{\{6.20 + (0.07 \times \cot 25^\circ 27' 30'')\} \tan 32^\circ 42' 45''}{(\tan 32^\circ 42' 45'' - \tan 25^\circ 27' 30'')} \text{ metre.}$$

$$= \frac{\{6.20 + (0.07 \times 2.10)\} \times 0.476083}{(0.6422967 - 0.476083)} \text{ metre.}$$

$$= \frac{6.347033131 \times 0.476083}{0.166213541} \text{ metre.}$$

$$= 18.180 \text{ metre.}$$

\therefore The total distance $D = (b + d) = (18.180 + 6.20)$ metre ; = 24.380 metre.

5. Height of the object above the line of collimation, $h_1 = D \tan \alpha_2$
= $(24.380 \times \tan 25^\circ 27' 30'')$ metre.
= (24.380×0.476083) metre.
= 11.607 metre.

\therefore The total Height of the object above the ground surface, $H = (h_1 + h_2)$
= $(11.607 + 1.45)$ metre.
= 13.057 metre.

PLOTTING OF **HEIGHT & DISTANCE**

BY
TRANSIT THEODOLITE

Base : Inaccessible

DATE : 28/8/2000

PLACE : Chandernagore College ground

TIME : 12 Noon To 1.40 P.M.

INSTRUMENT NO : 55607
OBJECT : Rain Water Pipe
SURVEYOR : Roll-No:404/AH-010027.
(Group-A)

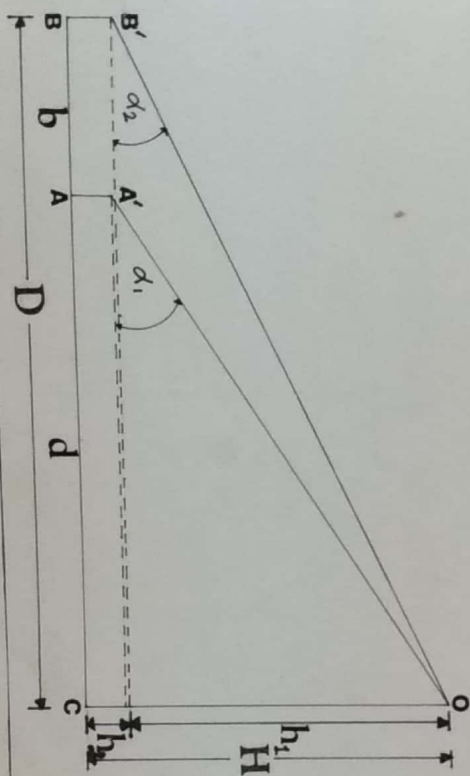
EXAMINED

Department of Geography,
WEEKANANDA MAHAVIDYALAYA
BURDWAN.

Index.

- $b \Rightarrow$ Known distance.
- $d \Rightarrow$ Unknown distance.
- $D \Rightarrow$ Total Ground distance.
- $h_1 \Rightarrow$ Height of the Object above the line of Collimation
- $h_2 \Rightarrow$ Collimation height
- $H \Rightarrow$ Total Height of the Object above Ground surface
- $AA' \Rightarrow$ Instrumental Height at Station-A
- Instrumental Height at Station-B
- $\angle \alpha_1 \Rightarrow 32^\circ 42' 45''$
- $\angle \alpha_2 \Rightarrow 25^\circ 27' 30''$

am
11/8/20



SCALE

Horizontal : $1:200$.

Vertical :



Field Book

DETERMINATION OF HEIGHT & DISTANCE

BY
TRANSIT THEODOLITE
Base : Inaccessible

DATE : 28/8/2000.

INSTRUMENT NO : 55607.

PLACE : CHANDERNAGORE COLLEGE GROUND.

OBJECT : RAIN WATER PIPE.

TIME : 12 NOON TO 1.40 P.M.

SURVEYOR : ROLL NO. : 404/AH-010027.
(GROUP-A).

STATION	FACE	VERNIER READING			REMARKS
		VC	VD	MEAN	
Station - 'A'	delt	32° 46'	32° 45'	32° 45' 30"	1. Instrumental height at station A & B = 1.38 metre. 2. Known distance = 62.0 metre.
	Right	32° 40'	32° 40'	32° 40'	
	GRAND MEAN ANGLE	—	—	32° 42' 45"	
	delt	25° 40'	25° 40'	25° 40'	
Station - 'B'	Right	25° 39'	25° 37'	25° 38'	3. <i>Signature</i>
	GRAND MEAN ANGLE	—	—	25° 39'	

CALCULATION OF HEIGHT & DISTANCE

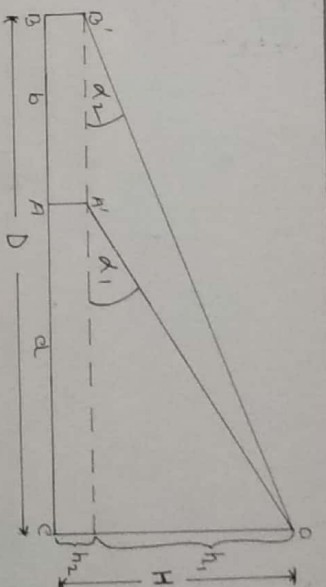
Base: Unaccessible.

1. Known distance, $b = 6.20$ metre.

2. $\alpha_1 = 32^\circ 42' 45''$; $\alpha_2 = 25^\circ 39'$.

3. Height of the instrument at both, [Same height]

Station A & B, $(h_2) = 1.38$ metre.



$$4. \text{Unknown distance, } d = \frac{b \tan \alpha_2}{\tan \alpha_1 - \tan \alpha_2} \quad ; \quad = \frac{6.20 \times \tan 25^\circ 39'}{\tan 32^\circ 42' 45'' - \tan 25^\circ 39'} \text{ metre.}$$

$$= \frac{6.20 \times 0.48019316}{0.6422967 - 0.48019316} \text{ metre.}$$

$$= \frac{2.977197594}{0.162103556} \text{ metre.}$$

$$= 18.366 \text{ metre.}$$

$$\therefore \text{The total ground distance, } D = (b + d) = (6.20 + 18.366) \text{ metre;} \\ = 24.566 \text{ metre.}$$

$$5. \text{Height of the object above the line of collimation, } h_1 = D \tan \alpha_2 = 24.566 \times \tan 25^\circ 39' \text{ metre.} \\ = (24.566 \times 0.48019316) \text{ metre} \\ = 11.796 \text{ metre.}$$

$$\therefore \text{Height of the object above ground surface, } H = (h_1 + h_2) = (11.796 + 1.38) \text{ metre} \\ = 13.176 \text{ metre.}$$

PLOTTING OF **HEIGHT & DISTANCE**

BY

TRANSIT THEODOLITE

Base : Inaccessible

DATE : 28/8/2000.

PLACE : CHANDERNAGORE COLLAGE GROUND.

TIME : 12.00. NOON TO 01.40 P.M.

INSTRUMENT NO : 55607.

OBJECT : RAIN WATER PIPE.

SURVEYOR : ROLL - NO. 404/AH - 010027.
(GROUP - A).

Department of Geography,
MEKANANDA MAHAVIDYALAYA
BURDWAN.

EXAMINED

★ Index ★

$b \Rightarrow$ Known Distance.

$d \Rightarrow$ Unknown Distance.

$D \Rightarrow$ Total Ground Distance.

$h_1 \Rightarrow$ Height Of the Object above

Line Of Collimation.

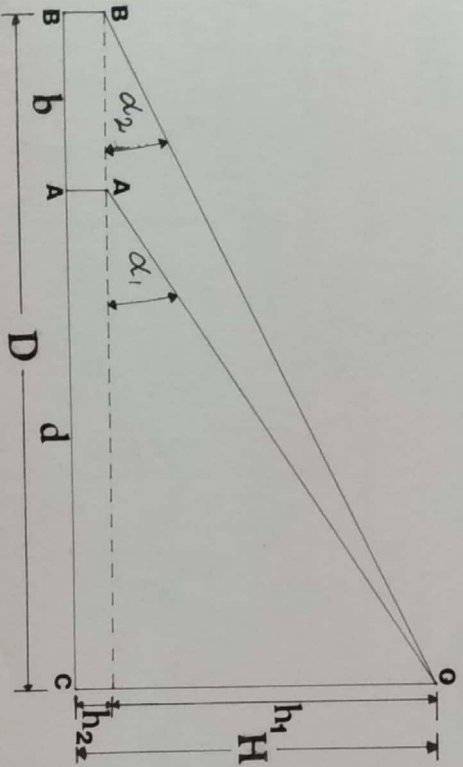
$h_2 \Rightarrow$ Collimation Height.

$H \Rightarrow$ Height Of the Object above.

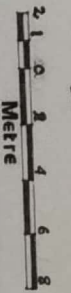
Ground Surface.

$\angle \alpha_1 \Rightarrow 32^\circ 42' 45''$.

$\angle \alpha_2 \Rightarrow 25^\circ 39''$.



Horizontal
Vertical
Scale
1 : 200



Drawn
11.6.2000

DETERMINATION OF HEIGHT & DISTANCE

by

TRANSIT THEODOLITE

Base : Inaccessible

- Date : 28/8/2000.
- Place : Chandernagore College ground.
- Time : 12 Noon to 1.40 P.M.

- Instrument No : 55607.
- Object : Rain water pipe.
- Surveyor : ROLL NO. : 404/AH - 010027 (Group-A).

STATION	FACE	VERNIER READING			REMARKS
		VC	VD	MEAN	
Station - A	Left	32°46'	32°45'	32°45'30"	1. Instrumental height at Station - A = 1.38 metre.
	Right	32°40'	32°40'	32°40'	
	GRAND MEAN ANGLE	—	—	32°42'45"	
Station - B	Left	25°45'	25°45'	25°45'30"	2. Instrumental height at Station - B = 1.26 metre.
	Right	25°45'	25°45'	25°45'	
	GRAND MEAN ANGLE	—	—	25°45'15"	

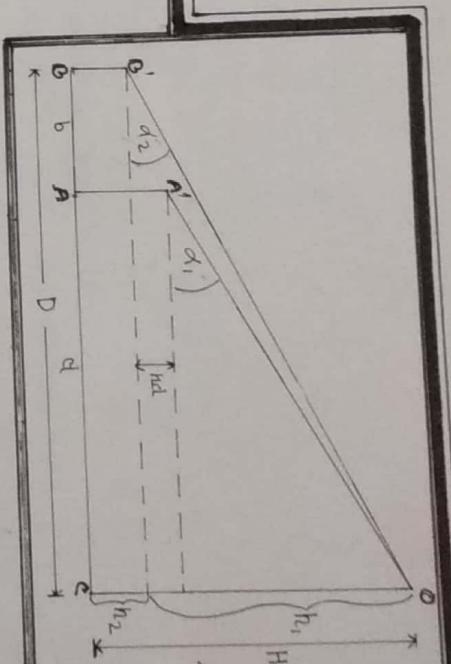
EXAMINED

Signature

CALCULATION

OF HEIGHT & DISTANCE

BASE: INACCESSIBLE



1. Known distance, $b = 6.20$ metre.

2. $\angle \alpha_1 = 32^\circ 42' 45''$; $\angle \alpha_2 = 25^\circ 45' 15''$

3. Height of the instrument at station A = 1.380 metre.

Height of the instrument at station B = 1.26 metre.

\therefore The collimation elevation, $h_{cl} = (1.38 - 1.26)$ metre; $= 0.12$ metre.

4. Unknown distance, $d = \frac{\{b - (h_{cl} \cot \alpha_2)\} \tan \alpha_2}{\tan \alpha_1 - \tan \alpha_2}$; $= \frac{\{6.20 - (0.12 \times \cot 25^\circ 45' 15'')\} \tan 25^\circ 45' 15''}{(\tan 32^\circ 42' 45'' - \tan 25^\circ 45' 15'')}$ metre.

$$= \frac{\{6.20 - (0.12 \times 2.07)\} \times 0.482432385}{0.642296716 - 0.482432385} \text{ metre.}$$

$$= \frac{2.871080788}{0.159864331} \text{ metre.}$$

$$= 17.959 \text{ metre.}$$

\therefore The total ground distance, $D = (b + d) = (6.20 + 17.959)$ metre $= 24.159$ metre.

5. Height of the object above the line of collimation, $h_1 = D \tan 25^\circ 45' 15'' = (24.159 \times 0.482432385)$ metre $= 11.655$ metre.

\therefore The total height of the object above the ground surface, $H = (h_1 + h_2) = (11.655 + 1.260)$ metre $= 12.916$ metre.

PLOTTING OF **HEIGHT & DISTANCE**

BY

TRANSIT THEODOLITE

Base : Inaccessible

★ DATE : 28/8/2000.

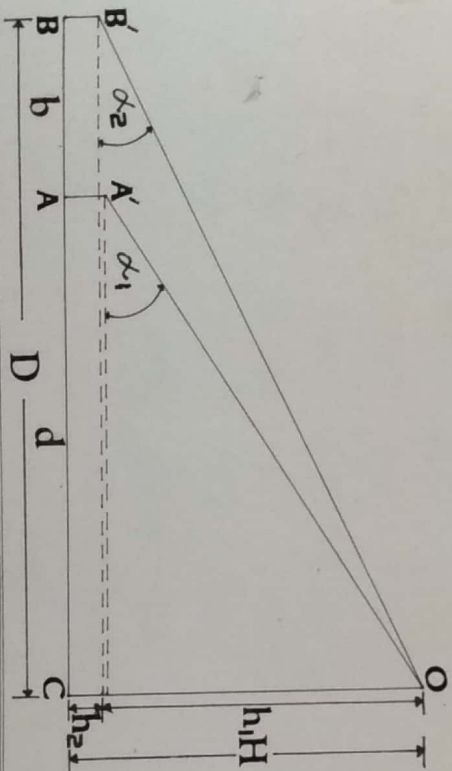
★ PLACE : CHANDERNAGORE COLLAGE GROUND.

★ TIME : 12.00 NOON TO 1.40 P.M.

★ INSTRUMENT NO : 55607.

★ OBJECT : RAIN WATER PIPE.

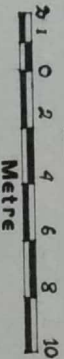
★ SURVEYOR : ROLL NO → 404/AH-010027.
(GROUP-A).



b → Known Distance.
 d → Unknown Distance.
 D → Total Ground Distance.
 h₁ → Height of the Object above,
 the Line of Collimation.
 h₂ → Collimation Height.
 H → Height of the Object above
 Ground Surface.
 AA' → Instrumental Height at Station-A.
 BB' → Instrumental Height at Station-B.
 α₁ → 32° 42' 45" //
 α₂ → 25° 45' 15" //

★ ১৭০২৪ ★

Scale
 Horizontal → 1 : 200
 Vertical → 1 : 200



১৭.০২৪